



SYSTEM AND PROCESS ASSESSMENT RESEARCH LABORATORY

**SPAR Lab**

Civil, Architectural and Environmental Engineering • 103/104-E Butler-Carlton Hall

# **SMART ROCK TECHNOLOGY FOR REAL-TIME MONITORING OF BRIDGE SCOUR AND RIPRAP EFFECTIVENESS – GUIDELINES AND VISUALIZATION**

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Technical Advisory Council Meeting No.2



# OUTLINE OF THIS PRESENTATION

- **Localization of Smart Rock**
  - Localization Algorithm
  - Experimental Validation at Bridge Site
- **Smart Rock Design and Prototyping**
  - Motion under Various Flow Conditions
  - Design Guidelines
  - Final Design
  - Prototyping with Concrete Encasement
- **Future Tasks**
  - Deployment Plan
  - Field Measurement Plan



# LOCALIZATION OF SMART ROCK

- **Localization Algorithm**

- The total magnetic field (intensity) of a smart rock with embedded magnet and its surrounding ferromagnetic substances is measured with a magnetometer at various points around the smart rock.
- The ambient magnetic field of the ferromagnetic substances is measured with the magnetometer and an orientation device at the same points.
- The coordinates of the measurement points are surveyed using a survey equipment ( Total Station).
- The intensity and coordinate measurements at six or more points enable the localization of the smart rock.



# LOCALIZATION OF SMART ROCK

- **Localization Algorithm (Cont.)**

- **Ambient Field in Global XYZ Coordinate System**

- ✓ *Surrounding ferromagnetic substances*

- ✓  $B_A$  = *ambient magnetic field vector at a measurement point Q (X,Y,Z)*

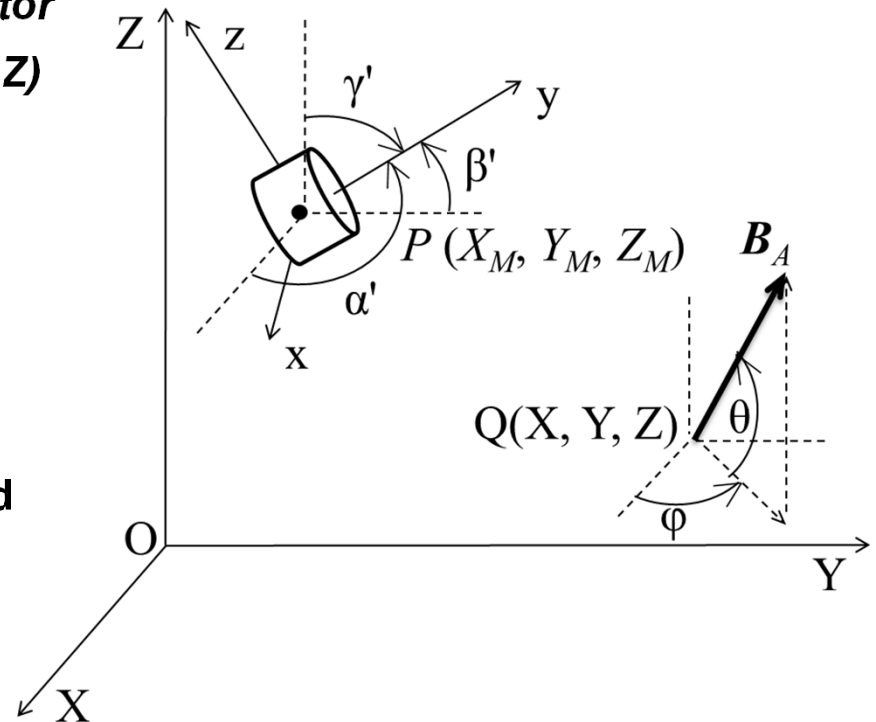
- ✓ *Three components of  $B_A$ :*

$$B_{XA} = B_A \cos \theta \cos \varphi$$

$$B_{YA} = B_A \cos \theta \sin \varphi$$

$$B_{ZA} = B_A \sin \theta$$

- ✓  $\theta(0, \pi)$  and  $\varphi(0, 2\pi)$  are measured from a custom-made orientation device



# LOCALIZATION OF SMART ROCK

## • Localization Algorithm (Cont.)

### – Magnetic Field of a Permanent Magnet in XYZ System

- ✓ *Cylindrical magnet*  $P(X_M, Y_M, Z_M)$
- ✓ *Orientation defined in local*  $xyz$  *coordinate system*
- ✓ *Three components of*  $B_M$  *at Point*  $Q$ :

$$\begin{pmatrix} B_{XM} \\ B_{YM} \\ B_{ZM} \end{pmatrix} = \mathbf{T}^{-1} \begin{pmatrix} k3xy / r^5 \\ k(2y^2 - x^2 - z^2) / r^5 \\ k3zy / r^5 \end{pmatrix}$$

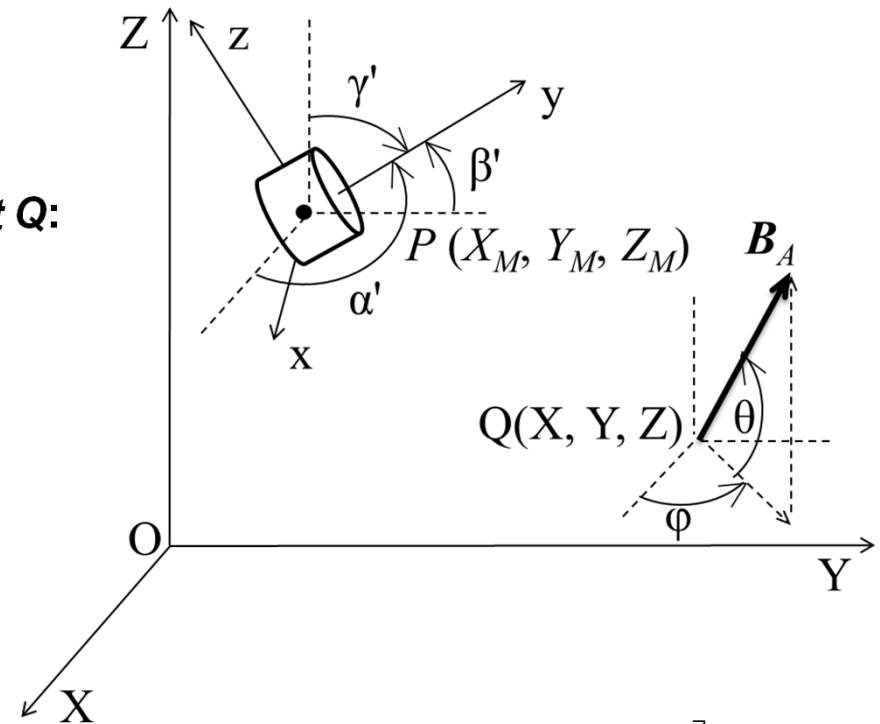
$$r = \sqrt{x^2 + y^2 + z^2}$$

$$x = a_{xX}(X - X_M) + a_{xY}(Y - Y_M) + a_{xZ}(Z - Z_M)$$

$$y = a_{yX}(X - X_M) + a_{yY}(Y - Y_M) + a_{yZ}(Z - Z_M)$$

$$z = a_{zX}(X - X_M) + a_{zY}(Y - Y_M) + a_{zZ}(Z - Z_M)$$

$$\mathbf{T} = \begin{bmatrix} a_{xX} & a_{xY} & a_{xZ} \\ a_{yX} & a_{yY} & a_{yZ} \\ a_{zX} & a_{zY} & a_{zZ} \end{bmatrix} = \begin{bmatrix} \cos \beta \cos \gamma & \cos \beta \sin \gamma & -\sin \beta \\ \sin \alpha \sin \beta \cos \gamma - \cos \alpha \sin \gamma & \sin \alpha \sin \beta \sin \gamma + \cos \alpha \cos \gamma & \sin \alpha \cos \beta \\ \cos \alpha \sin \beta \cos \gamma + \sin \alpha \sin \gamma & \cos \alpha \sin \beta \sin \gamma - \sin \alpha \cos \gamma & \cos \alpha \cos \beta \end{bmatrix}$$





# LOCALIZATION OF SMART ROCK

- **Localization Algorithm (Cont.)**

- **Total Magnetic Field at Point Q in XYZ System**

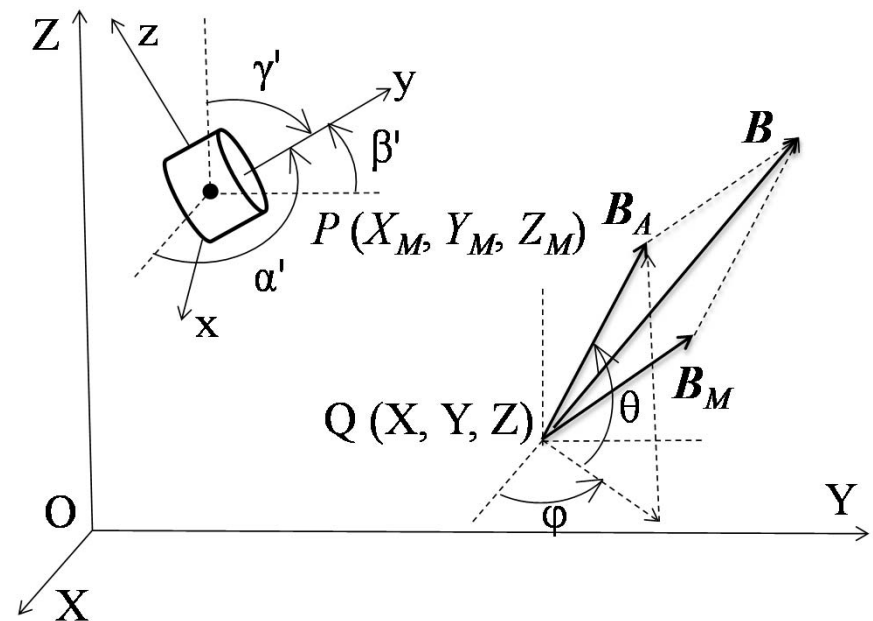
- ✓ *Total magnetic field intensity:*

$$B = \sqrt{(B_{XM} + B_{XA})^2 + (B_{YM} + B_{YA})^2 + (B_{ZM} + B_{ZA})^2}$$

- ✓  $B = B(B_A, \theta, \varphi, k, X, Y, Z, X_M, Y_M, Z_M, \alpha, \beta, \gamma)$  at any measurement point Q (X,Y,Z)

- ✓ Given  $k, \theta, \varphi, B_A, X, Y, Z, B = B(X_M, Y_M, Z_M, \alpha, \beta, \gamma)$

- ✓ Minimum measurements at six points



# LOCALIZATION OF SMART ROCK

- **Localization Algorithm (Cont.)**

- **Unknown Orientation**

✓ *SRSS error between predicted intensity  $B_i^{(P)}$  and the measured intensity  $B_i^{(M)}$  at  $n$  measurement points*

$$J(X_M, Y_M, Z_M, \alpha, \beta, \gamma) = \sqrt{\sum_{i=1}^n [B_i^{(P)} - B_i^{(M)}]^2}$$

$$\frac{\partial J(X_M, Y_M, Z_M, \alpha, \beta, \gamma)}{\partial X_M} = 0$$

$$\frac{\partial J(X_M, Y_M, Z_M, \alpha, \beta, \gamma)}{\partial Y_M} = 0$$

$$\frac{\partial J(X_M, Y_M, Z_M, \alpha, \beta, \gamma)}{\partial Z_M} = 0$$

$$\frac{\partial J(X_M, Y_M, Z_M, \alpha, \beta, \gamma)}{\partial \alpha} = 0$$

$$\frac{\partial J(X_M, Y_M, Z_M, \alpha, \beta, \gamma)}{\partial \beta} = 0$$

$$\frac{\partial J(X_M, Y_M, Z_M, \alpha, \beta, \gamma)}{\partial \gamma} = 0$$

- **Known Orientation ( $\alpha=0, \beta=0, \text{ and } \gamma=0$ )**

$$J(X_M, Y_M, Z_M) = \sqrt{\sum_{i=1}^n [B_i^{(P)} - B_i^{(M)}]^2}$$

$$\frac{\partial J(X_M, Y_M, Z_M)}{\partial Y_M} = 0$$

$$\frac{\partial J(X_M, Y_M, Z_M)}{\partial Z_M} = 0$$

$$\frac{\partial J(X_M, Y_M, Z_M)}{\partial X_M} = 0$$



# LOCALIZATION OF SMART ROCK

- **Experimental Validation at Bridge Site**
  - Gasconade River Bridge Site, MO
  - **Two Smart Rocks**
    - ✓ *Unknown orientation: Arbitrarily Oriented System (AOS)*
    - ✓ *Known orientation: Automatically Pointing South System (APSS)*



(a) AOS



(b) APSS





# LOCALIZATION OF SMART ROCK

- **Experimental Validation at Bridge Site (Cont.)**
  - *Evaluation of  $k$ ,  $B_A$ ,  $\theta$  and  $\varphi$* 
    - ✓  *$K(\text{AOS}) = 41890.13(\text{nT}\cdot\text{m}^3)$ , measured with high precision level*
    - ✓  *$K(\text{APSS}) = 42542.27(\text{nT}\cdot\text{m}^3)$ , measured with low precision level*
    - ✓ *Ambient magnetic field lines are no longer in parallel due to ferromagnetic substances (e.g. reinforcement in bridge pier and deck)*
    - ✓ *Three parameters ( $B_A$ ,  $\theta$  and  $\varphi$ ) define the ambient magnetic field for each measurement point in space*
      - The field intensity  $B_A$  was measured with a magnetometer.
      - An Ambient Magnetic Field Orientation Device (AMFOD) was developed and prototyped to measure the angles  $\theta$  and  $\varphi$ .

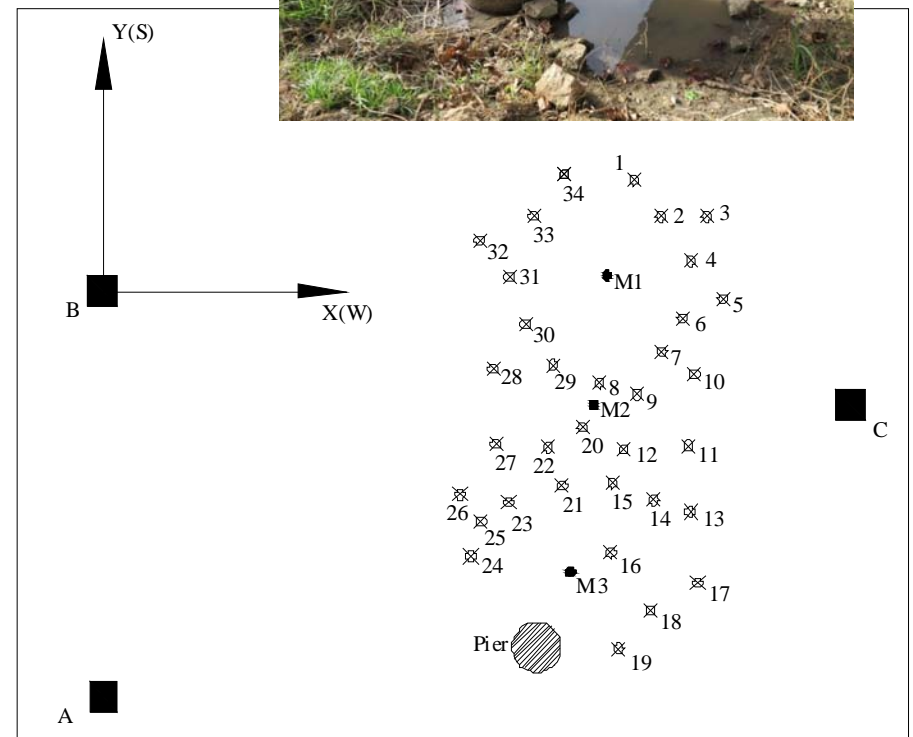


# LOCALIZATION OF SMART ROCK

- **Experimental Validation at Bridge Site (Cont.)**

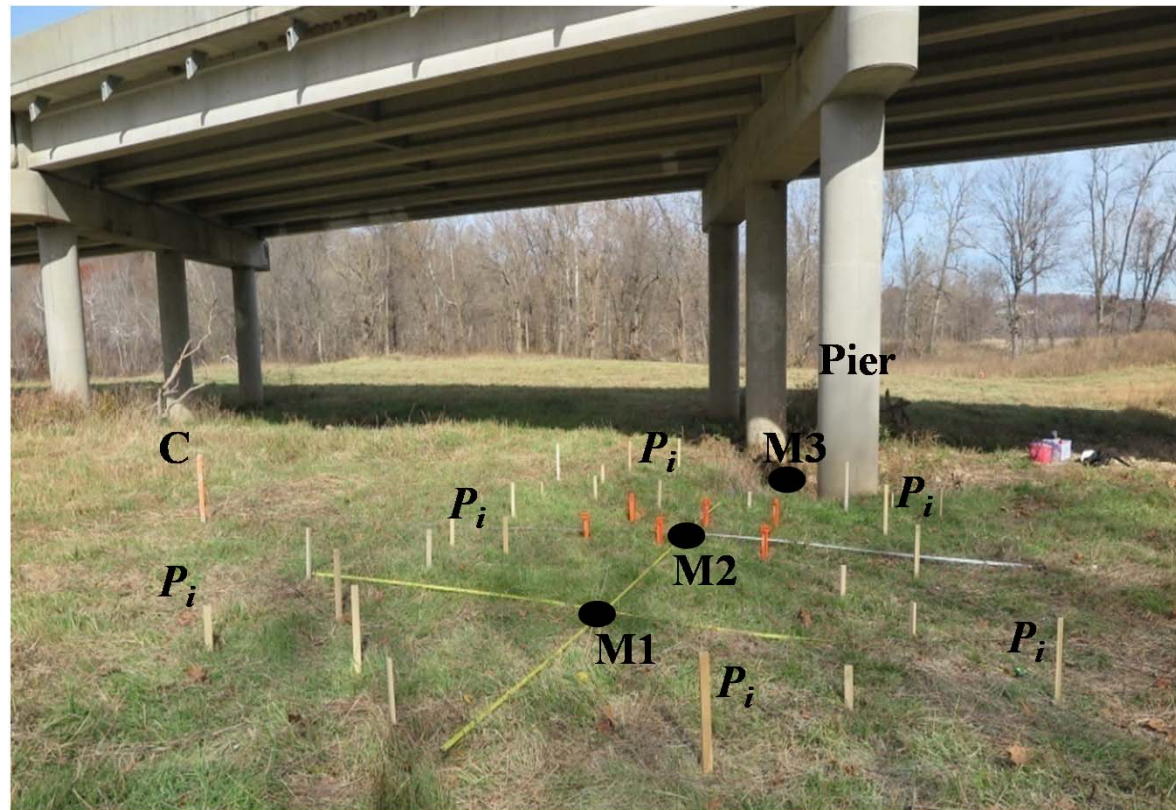
- **Test Setup around a Scour Hole**

- ✓ **Three magnet locations M1, M2, and M3 for AOS and APSS**
- ✓ **Total 34 measurement points**
- ✓ **Total Station at Point B to survey coordinates of three smart rocks and 34 sensor positions or measurement points**
- ✓ **AMFOD was set at the 34 points to measure  $\theta$  and  $\varphi$**



# LOCALIZATION OF SMART ROCK

- Experimental Validation of at Bridge Site (Cont.)
  - *Test Setup*



# LOCALIZATION OF SMART ROCK

- **Experimental Validation of at Bridge Site (Cont.)**

- ***Test Procedure***

- ✓ ***Step 1: Set the Global XYZ Coordinate System***

- ✓ ***Step 2: Select the Locations of Smart Rocks and the Sensor Head***

- *Smart rocks located far away from, near, and close to the bridge pier*
- *34 points distributed around M1, M2 and M3 bounded by circles with diameter of 1.5 m and 5 m around the pier*

- ✓ ***Step 3: Select a Calibration Point C for AMFOD***

- *Together with a fixed object as a reference to assist in determination of angle  $\varphi$*
- *Set away from the 34 measurement points to ensure the line of sight from laser light to Point C*





# LOCALIZATION OF SMART ROCK

- **Experimental Validation of at Bridge Site (Cont.)**
  - *Test Procedure (Cont.)*
    - ✓ *Step 4: Determine the Coordinates of Smart Rocks, Sensor Head and Calibration Point*



**Total Station and Prism for Positioning**





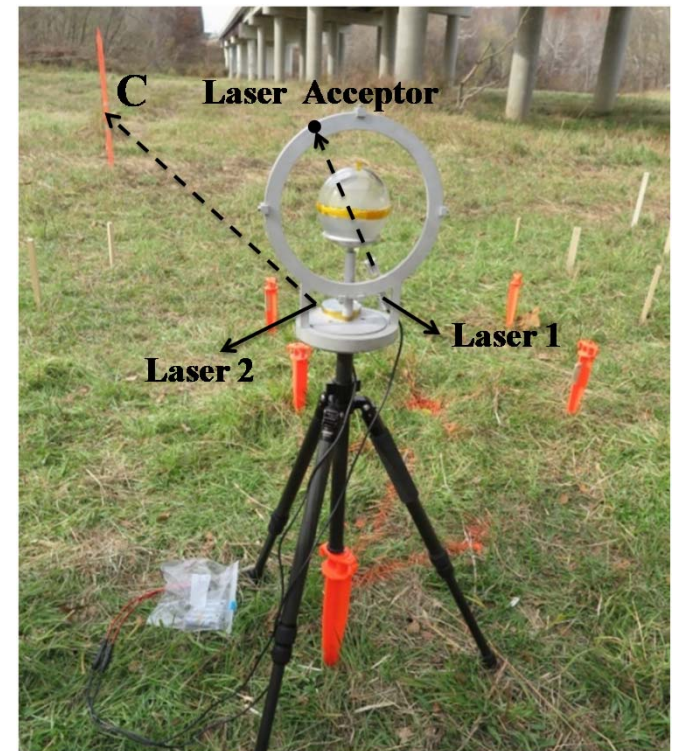
# LOCALIZATION OF SMART ROCK

- **Experimental Validation of at Bridge Site (Cont.)**

- ***Test Procedure (Cont.)***

- ✓ **Step 5: Measure  $\theta$  and  $\varphi$**

- *AMFOD placed at measurement point*
- *The center of high precision APSS kept along extension line of the orange plastic pole.*
- *Shooting light of Horizontal Laser 2 hits on the wooden pole at Point C*
- *Inside magnet automatically aligned to the ambient magnetic field*
- *Shooting light of Laser 1 goes through the hole at the center line of APSS and hits on the center of laser acceptor*
- *Read  $\theta$  and  $\varphi$*



**AMFOD Setup and Operational Mechanism**



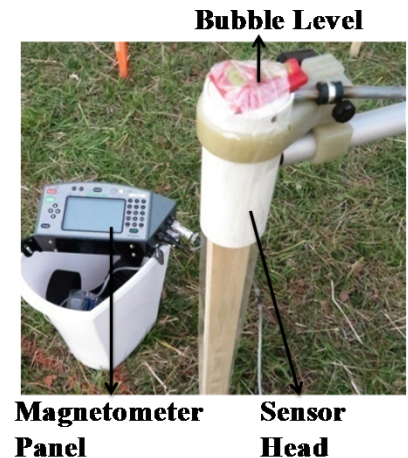
# LOCALIZATION OF SMART ROCK

## • Experimental Validation of at Bridge Site (Cont.)

### – Test Procedure (Cont.)

#### ✓ Step 6: Measure the Ambient Magnetic Field Intensity

- Level bubble attached on the sensor head ensures the sensor perpendicular to the ground
- Keep the center of the sensor head consistent with that of the high precision APSS by a 57.7 cm wooden pole
- Conduct measurement without vehicles
- At least three measurements to ensure accuracy and repeatability



Magnetometer Setup and Operation





# LOCALIZATION OF SMART ROCK

- **Experimental Validation of at Bridge Site (Cont.)**
  - *Test Procedure (Cont.)*
    - ✓ *Step 7 & 8: Measure the Total Magnetic Field Intensity of AOS and APSS at M1, M2 and M3*



(a)  $M1_{APSS}$  or  $M2_{APSS}$



(b) APSS at M3



(c)  $M1_{AOS}$  or  $M2_{AOS}$



(d) AOS at M3



# LOCALIZATION OF SMART ROCK

- **Experimental Validation at Bridge Site (Cont.)**
  - *Test Results*

**Table 1 Sensor Coordinates and Ambient Magnetic Field Intensities**

Measurement Point	Sensor Coordinates			Ambient Magnetic Field					
				Direction		Intensity			
	X/m	Y/m	Z/m	$\theta$ / rad	$\varphi$ / rad	$B_A$ /nT	$B_{AX}$ /nT	$B_{AY}$ /nT	$B_{AZ}$ /nT
C	15.284	-2.264	N/A	N/A	N/A	N/A	N/A	N/A	N/A
P1	10.882	2.202	-0.547	1.213	1.503	50798	1213	17748	47581
P2	11.425	1.481	-0.454	1.222	1.525	51417	810	17567	48316
P3	12.365	1.479	-0.576	1.222	1.477	51363	1637	17491	48266
P4	12.040	0.587	-0.483	1.197	1.485	51366	1603	18674	47825
P5	12.701	-0.160	-0.512	1.196	1.512	51296	1102	18768	47727
...	...	...	...	...	...	...	...	...	...
P33	8.813	1.487	-0.724	1.143	1.511	51330	1262	21249	46709
P34	9.455	2.322	-0.436	1.162	1.410	51417	3264	20158	47188



# LOCALIZATION OF SMART ROCK

- **Experimental Validation at Bridge Site (Cont.)**
  - *Test Results ( $M1_{APSS}$ )*

Location of Sensor Head	X(m)	Y(m)	Z(m)	$B_i^{(M)}$ (nT)
P1	10.882	2.202	-0.517	58120
P2	11.425	1.481	-0.424	56946
...	...	...	...	...
P9	10.940	-2.065	-0.687	52055
P10	12.119	-1.657	-0.665	50942
P20	9.822	-2.717	-0.665	53002
P28	7.989	-1.553	-0.659	51464
P29	9.216	-1.476	-0.661	51031
P30	8.651	-0.664	-0.757	48911
...	...	...	...	...
P34	9.455	2.322	-0.406	56421
<b>Predicted APSS Location <math>M1_{APSS}</math></b>	<b>10.249</b>	<b>0.454</b>	<b>-1.352</b>	N/A
<b>Measured APSS Location <math>M1_{APSS}</math></b>	<b>10.326</b>	<b>0.305</b>	<b>-1.407</b>	
<b>Location Prediction Error for <math>M_{APSS}</math></b>	<b>-0.077</b>	<b>0.149</b>	<b>0.055</b>	
SRSS Error in Coordinate	0.176 m			





# LOCALIZATION OF SMART ROCK

- **Experimental Validation at Bridge Site (Cont.)**
  - *Test Results ( $M3_{APSS}$ )*

Location of Sensor Head	X(m)	Y(m)	Z(m)	$B_i^{(M)}$ (nT)
P9	10.940	-2.065	-0.657	52766
P11	11.991	-3.082	-0.558	52422
P12	10.670	-3.162	-0.670	55203
...	...	...	...	...
P20	9.822	-2.717	-0.635	55164
P21	9.413	-3.877	-0.748	63734
P23	8.313	-4.215	-0.501	59204
P25	7.750	-4.591	-0.858	58350
P26	7.315	-4.055	-0.726	56087
P27	8.043	-3.046	-0.553	55198
<b>Predicted APSS Location <math>M3_{APSS}</math></b>	<b>9.527</b>	<b>-5.520</b>	<b>-1.850</b>	N/A
<b>Measured APSS Location <math>M3_{APSS}</math></b>	<b>9.576</b>	<b>-5.584</b>	<b>-1.822</b>	
<b>Location Prediction Error for <math>M3_{APSS}</math></b>	<b>-0.049</b>	<b>0.064</b>	<b>-0.028</b>	
SRSS Error in Coordinate	0.085m			



# LOCALIZATION OF SMART ROCK

- **Experimental Validation at Bridge Site (Cont.)**
  - *Test Results ( $M1_{AOS}$ )*

Location of Sensor Head	X(m)	Y(m)	Z(m)	$B_i^{(M)}$ (nT)
P1	10.882	2.202	-0.517	53558
P2	11.425	1.481	-0.424	52767
...	...	...	...	...
P9	10.940	-2.065	-0.687	49665
P10	12.119	-1.657	-0.665	49567
P20	9.822	-2.717	-0.665	51538
P28	7.989	-1.553	-0.659	50607
P29	9.216	-1.476	-0.661	48149
P30	8.651	-0.664	-0.757	47696
...	...	...	...	...
P34	9.455	2.322	-0.406	54539
<b>Predicted AOS Location <math>M1_{AOS}</math></b>	<b>10.265</b>	<b>0.235</b>	<b>-1.456</b>	N/A
<b>Measured AOS Location <math>M1_{AOS}</math></b>	<b>10.326</b>	<b>0.305</b>	<b>-1.422</b>	
<b>Location Prediction Error for <math>M1_{AOS}</math></b>	<b>-0.061</b>	<b>-0.070</b>	<b>-0.034</b>	
SRSS Error in Coordinate	0.099 m			



# LOCALIZATION OF SMART ROCK

- **Experimental Validation at Bridge Site (Cont.)**
  - *Test Results ( $M3_{AOS}$ )*

Location of Sensor Head	X(m)	Y(m)	Z(m)	$B_i^{(M)}$ (nT)
P9	10.940	-2.065	-0.667	52651
P12	10.670	-3.162	-0.680	54660
P13	12.031	-4.399	-0.745	52095
...	...	...	...	...
P20	9.822	-2.717	-0.645	54929
P21	9.413	-3.877	-0.758	62508
P23	8.313	-4.215	-0.511	59364
P25	7.750	-4.591	-0.868	59523
P26	7.315	-4.055	-0.736	56642
P27	8.043	-3.046	-0.563	55399
<b>Predicted AOS Location <math>M3_{AOS}</math></b>	<b>9.514</b>	<b>-5.519</b>	<b>-1.860</b>	N/A
<b>Measured AOS Location <math>M3_{AOS}</math></b>	<b>9.576</b>	<b>-5.584</b>	<b>-1.837</b>	
<b>Location Prediction Error for <math>M3_{AOS}</math></b>	<b>-0.062</b>	<b>0.065</b>	<b>-0.023</b>	
SRSS Error in Coordinate	0.093m			



# SMART ROCK DESIGN AND PROTOTYPING

- **Motion under Various Flow Conditions**
  - ***Criteria of Incipient Motion of Rocks***

- ✓ ***Critical velocity (HEC 18, 3<sup>rd</sup> version)***

$$V_c = \frac{K_s^{1/2} (S_s - 1)^{1/2} d^{1/2} y^{1/6}}{n}$$

- ✓ ***Critical shear stress(HEC18, 3<sup>rd</sup> version)***

$$\tau_c = K_s (\rho_s - \rho_w) g d$$

$$\tau_{local} = \left( \frac{n V_{local}}{K_u} \right)^2 \frac{\gamma_w}{y^{1/3}}$$

- ✓ ***Riprap size design(HEC 23)***

$$D_{50} = \frac{0.692(KV)^2}{2g(S_s - 1)}$$



# SMART ROCK DESIGN AND PROTOTYPING

- **Motion under Various Flow Conditions (Cont.)**
  - *Incipient Motion at Various Bridge Sites*
    - ✓ *Highway 1 over Waddell Creek (Br. No. 36-0065)*



Highway No.1 Waddell Creek Bridge





# SMART ROCK DESIGN AND PROTOTYPING

- **Motion under Various Flow Conditions(Cont.)**
  - ***Incipient Motion at Various Bridge Sites(Cont.)***
    - ✓ ***Highway 1 over Waddell Creek (Br. No. 36-0065)(Cont.)***
      - *Located about 17miles north of the city of Santa Cruz*
      - *Build in 1947, 4-span structure with total 180.8ft long and 31.7ft wide*
      - *Continuous reinforced concrete (RC) T-girders supported on RC piers and seat-type abutments*
      - *Up stream of the bridge, small mountain dominates the terrain; down stream, the channel alignment changes with flow intensity towards the Pacific Ocean*
      - *In Feb,2000, high flows from a storm caused severe erosion to the upstream channel banks and extending to the embankment at Abutment 1. Some piles at Pier 2 was exposed.*
      - *Then, classified as scour critical*



# SMART ROCK DESIGN AND PROTOTYPING

- **Motion under Various Flow Conditions(Cont.)**
  - ***Incipient Motion at Various Bridge Sites(Cont.)***

- ✓ ***Highway 1 over Waddell Creek (Br. No. 36-0065)(Cont.)***

- *The 100-year flood discharge (Q100) is 170 m<sup>3</sup>/s estimated from the regional flood-frequency equation based on the historical gage data from USGS.*
- *High water elevation level( HWEL) is 2.865 m during 100-year flood.*
- *The flow depth (y) and velocity (V) in the directly upstream of various piers is:*

Bent No.	2	3	4
y (m)	3.566	2.012	0.152
V (m/s)	2.286	3.048	1.585

- *Select Bent 2 for calculation because of its unstable during 100-year flood provided by Caltrans*



# SMART ROCK DESIGN AND PROTOTYPING

- **Motion under Various Flow Conditions(Cont.)**
  - ***Incipient Motion at Various Bridge Sites(Cont.)***
    - ✓ ***Highway 1 over Waddell Creek (Br. No. 36-0065)(Cont.)***

- *Based on critical velocity (Bent 2)*

$$2.286 = \frac{0.052^{1/2} \left( \frac{\rho_s}{1000} - 1 \right)^{1/2} 0.25^{1/2} 3.566^{1/6}}{0.0325}, \quad \rho_s = 1278 \text{ kg / m}^3$$

Where,

$K_s = 0.052$  for fine cobbles from the USGS Scientific Investigations Report 2008-5093;

$S_s = \rho_s / 1000$  where  $\rho_s$  is the mass density of smart rocks in  $\text{kg/m}^3$ ;

$g = 9.81 \text{ m/s}^2$ ;

$d = 0.25 \text{ m}$  for smart rocks based on the required space for magnet embedment;

$V_c = V = 2.286 \text{ m/s}$  at Bent 2;

$y = 3.566 \text{ m}$  at Bent 2;

$n = 0.041d^{1/6} = 0.0325$ .



# SMART ROCK DESIGN AND PROTOTYPING

- **Motion under Various Flow Conditions(Cont.)**
  - ***Incipient Motion at Various Bridge Sites(Cont.)***

✓ ***Highway 1 over Waddell Creek (Br. No. 36-0065)(Cont.)***

- ***Based on riprap size (Abutment 5)***

$$0.25 = \frac{0.692(1.7 \times 1.585)^2}{2 \times 9.81 \left( \frac{\rho_s}{1000} - 1 \right)}, \quad \rho_s = 2024 \text{ kg / m}^3$$

Where,

$$D_{50} = 0.25 \text{ m};$$

$K=1.7$  for a rectangle pier;

$V = 1.585 \text{ m/s}$  at Bent 4;

$S_s = \rho_s/1000$  in  $\text{kg/m}^3$ ; and

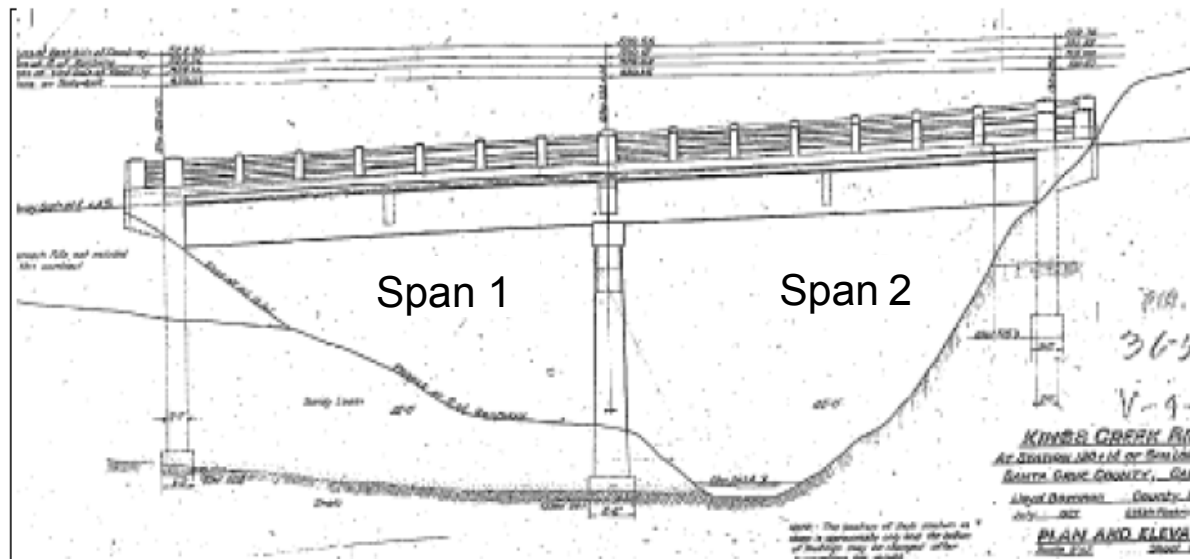
$$g = 9.81 \text{ m/s}^2.$$





# SMART ROCK DESIGN AND PROTOTYPING

- **Motion under Various Flow Conditions(Cont.)**
  - *Incipient Motion at Various Bridge Sites(Cont.)*
    - ✓ *Highway 9 over Kings Creek (Bridge No.36-0054)*



Schematic view of Kings Creek Bridge No.36-0054



# SMART ROCK DESIGN AND PROTOTYPING

- **Motion under Various Flow Conditions(Cont.)**
  - ***Incipient Motion at Various Bridge Sites(Cont.)***
    - ✓ ***Highway 9 over Kings Creek (Bridge No.36-0054)(Cont.)***
      - *2-span structures in Santa Cruz County over the Kings Creek*
      - *Located at the apex of a bend, main channel flow under span 2*
      - *Classified as scour critical in 2004 and footing pads at Bent 2 were exposed*
      - *A 2D hydraulic model of the flow was established by Caltrans to determine hydraulic parameter*
      - *The 100-year flood discharge (Q100) is 76.693 m<sup>3</sup>/s.*
      - *The flow depth (y) and velocity (V) was estimated as 0.3 m/s and 0.18m, respectively, at Bent 2.*



# SMART ROCK DESIGN AND PROTOTYPING

- **Motion under Various Flow Conditions(Cont.)**
  - ***Incipient Motion at Various Bridge Sites(Cont.)***

✓ ***Highway 1 over Waddell Creek (Br. No. 36-0065)(Cont.)***

- *Based on critical velocity (Bent 2)*

$$0.2 = \frac{0.052^{1/2} \left( \frac{\rho_s}{1000} - 1 \right)^{1/2} 0.25^{1/2} 0.18^{1/6}}{0.0325}, \quad \rho_s = 1006 \text{ kg / m}^3$$

*Where,*

$K_s = 0.052$  for fine cobbles from the USGS Scientific Investigations Report 2008-5093;

$S_s = \rho_s/1000$  where  $\rho_s$  is the mass density of smart rocks in  $\text{kg/m}^3$ ;

$g = 9.81 \text{ m/s}^2$ ;

$d = 0.25 \text{ m}$  for smart rocks based on the required space for magnet embedment;

$V_c = V = 0.2 \text{ m/s}$  at Bent 2;

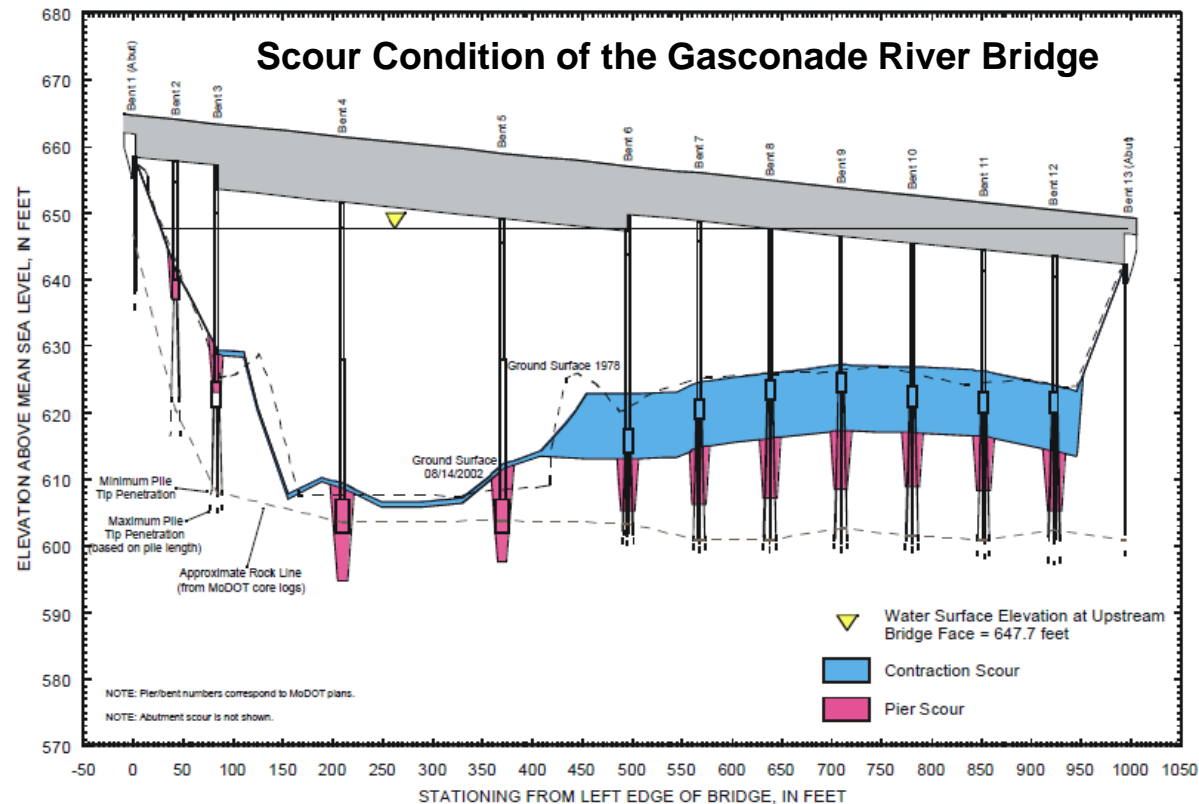
$y = 0.18 \text{ m}$  at Bent 2;

$n = 0.041d^{1/6} = 0.0325$ .



# SMART ROCK DESIGN AND PROTOTYPING

- Motion under Various Flow Conditions(Cont.)
  - *Incipient Motion at Various Bridge Sites(Cont.)*
    - ✓ *US63 Gasconade River Bridge*





# SMART ROCK DESIGN AND PROTOTYPING

- **Motion under Various Flow Conditions(Cont.)**
  - ***Incipient Motion at Various Bridge Sites(Cont.)***
    - ✓ ***US63 Gasconade River Bridge(Cont.)***
      - *Located approximately 5.5 miles southeast of Vienna in Maries County, MO.*
      - *Built in 1970's, 12-span concrete-girder Structures.*
      - *Bent 4 is potentially scour critical.*
      - *The 100-year flood discharge( $Q_{100} = 146000 \text{ cfs} = 4234 \text{ m}^3/\text{s}$ ) estimated from historical data recorded from USGS gage station at Jerome, MO( gage No. 06933500).*
      - *The cross sectional area (A) was estimated to be  $36544 \text{ ft}^2$  ( $3395 \text{ m}^2$ ).*



# SMART ROCK DESIGN AND PROTOTYPING

## • Motion under Various Flow Conditions(Cont.)

### – Incipient Motion at Various Bridge Sites(Cont.)

#### ✓ US63 Gasconade River Bridge(Cont.)

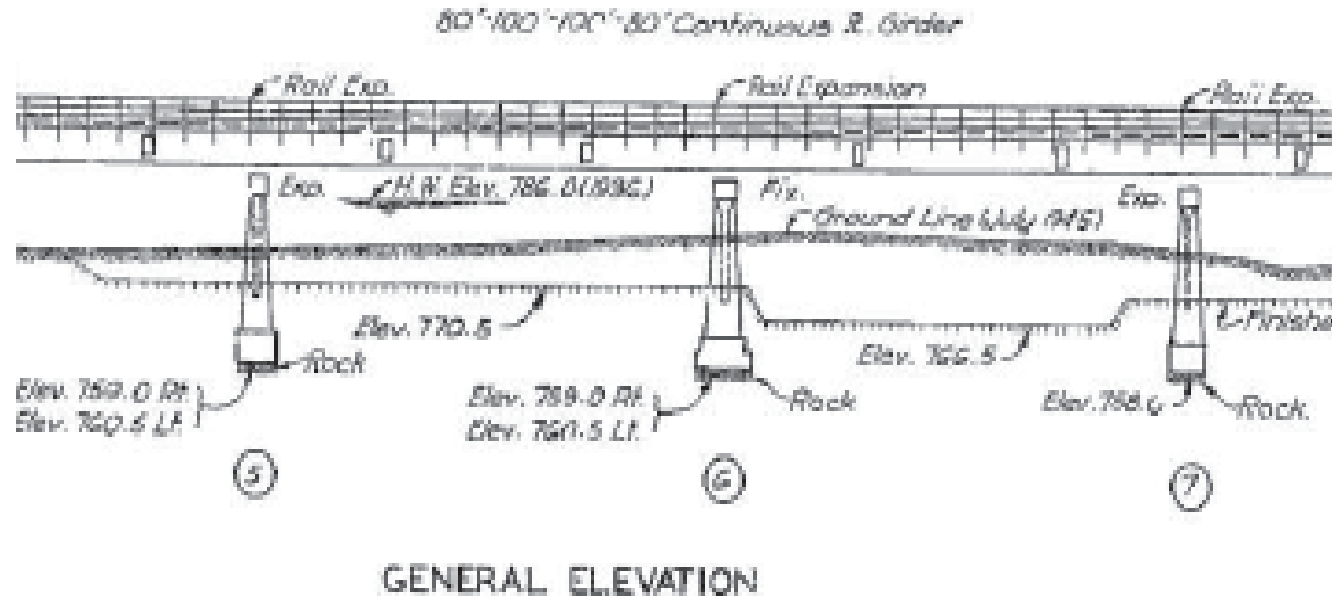
- The average channel velocity,  $V_{average} = Q_{100} / A = 1.218 \text{ m/s}$ .
- The velocity upstream of bent 4,  $V = 1.7 V_{average}$  considering pier in the main current of flow around a bend.
- Flow depth at Bent 4 is approximately 40ft (12.192m).
- Therefore, with same size of 0.25m, the density is:

$$1.218 \times 1.7 = \frac{0.052^{1/2} \left( \frac{\rho_s}{1000} - 1 \right)^{1/2} 0.25^{1/2} 12.192^{1/6}}{0.0325}, \quad \rho_s = 1151 \text{ kg} / \text{m}^3$$



# SMART ROCK DESIGN AND PROTOTYPING

- Motion under Various Flow Conditions(Cont.)
  - *Incipient Motion at Various Bridge Sites(Cont.)*
    - ✓ I-44 Roubidoux Creek Bridge (Bridge No.L0039)



Schematic view of I-44 Roubidoux Creek Bridge at Bents 5-7



# SMART ROCK DESIGN AND PROTOTYPING

- **Motion under Various Flow Conditions(Cont.)**
  - ***Incipient Motion at Various Bridge Sites(Cont.)***
    - ✓ ***I-44 Roubidoux Creek Bridge (Bridge No.L0039)(Cont.)***
      - *Located about 12 miles south of Crocker in Pulaski County, MO.*
      - *10-spans with main flow going between Bents 5 and 7*
      - *Bent 6 is potentially scour critical.*
      - *The maximum discharge and flow depth ( $Q_{max} = 18200 \text{ cfs} = 515.4 \text{ m}^3/\text{s}$  and  $y=18.70 \text{ ft} = 5.70 \text{ m}$ ) recorded at the USGS gage station( USGS 0698300, Roubidoux Creek above Fort Leonard Wood, MO).*
      - *The cross sectional area (A) was estimated to be  $11703 \text{ ft}^2$  ( $1087 \text{ m}^2$ ).*





# SMART ROCK DESIGN AND PROTOTYPING

- **Motion under Various Flow Conditions(Cont.)**

- ***Incipient Motion at Various Bridge Sites(Cont.)***

- ✓ ***I-44 Roubidoux Creek Bridge (Bridge No.L0039)(Cont.)***

- *The average channel velocity,  $V_{average} = Q_{max} / A = 0.474$  m/s.*
      - *The velocity upstream of bent 4,  $V=1.7 V_{average}$  considering pier in the main current of flow around a bend.*
      - *Therefore, with same size of 0.25m, the density is:*

$$0.474 \times 1.7 = \frac{0.052^{1/2} \left( \frac{\rho_s}{1000} - 1 \right)^{1/2} 0.25^{1/2} 5.70^{1/6}}{0.0325}, \quad \rho_s = 1030 \text{ kg / m}^3$$



# SMART ROCK DESIGN AND PROTOTYPING

- **Design Guidelines of Smart Rocks**

- ***Introduction***

- ✓ *Passive smart rocks embedded with permanent magnets, and remotely located with one or several magnetometers*
    - ✓ *Active smart rocks embedded with electronic device, and located from a remote measurement through wireless communication*
    - ✓ *Properly designed smart rocks*
    - ✓ *Onset movement of riprap slope protection*
    - ✓ *Maximum scour depth*



# SMART ROCK DESIGN AND PROTOTYPING

- **Design Guidelines of Smart Rocks**

- ***Design Considerations***

- ✓ ***Meet two requirements***

- *Facilitate remote measurement for rock localization*
      - *Ensure automatic movement to the bottom of a scour hole to be monitored*

- ✓ ***The size of smart rock is constrained by minimum size of permanent magnet***

- ✓ ***Always stay at the river bed***

- ✓ ***Overcome water current and roll down the slope of a scour hole***

- ✓ ***Remain at the bottom of the hole***

- ✓ ***Density of smart rocks range from that of water and rocks***

- ✓ ***Size and density highly depend on critical velocity and depth of water flow***

- ✓ **Use**  $d = \frac{(nV_c)^2}{K_s y^{1/3} (S_s - 1)}$  **and**  $D_{50} = \frac{0.692(KV)^2}{2g(S_s - 1)}$



# SMART ROCK DESIGN AND PROTOTYPING

- **Design Guidelines of Smart Rocks**

- ***Design Procedure***

- ✓ ***Step 1: Determine hydraulics parameters near a bridge site***

- *Flow velocity and water depth directly upstream of piers for 100-year flood*
      - *Collected from hydraulic studies by USGS or FEMA*
      - *Estimated from the data recorded by USGS gage station*

- ✓ ***Step 2: Constrain the size and density of a smart rock***

- *Inversely proportional relation between size and density*
      - *Given density, find out the size*
      - *Given size, find out density (preferred because of the embedded object)*

- ✓ ***Step 3: Finalize the design of smart rocks***

- *Multiply by a design factor(1.2-1.3) to account for any potential errors*
      - *Consider the easy of deployment and fabrication*





# SMART ROCK DESIGN AND PROTOTYPING

- **Final Design of Smart Rocks**

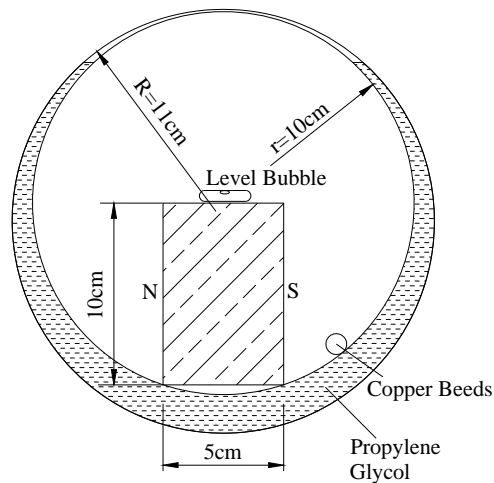
- ***Size and Density***

- ✓ *Diameter of 0.25m based on standard mold size*
    - ✓ *Multiply by 1.2 or 1.3 times to avoid washing away*
    - ✓ *Highway 1 Waddell Creek Bridge:  $1.2 \times 1278 = 1530 \text{ kg/m}^3$*
    - ✓ *Highway 9 Kings Creek Bridge:  $1.3 \times 1006 = 1308 \text{ kg/m}^3$*
    - ✓ *US63 Gasconade River Bridge:  $1.3 \times 1151 = 1496 \text{ kg/m}^3$*
    - ✓ *I-44 Roubidoux Creek Bridge:  $1.3 \times 1030 = 1339 \text{ kg/m}^3$*
    - ✓ *The target density of smart rocks:  $1530 \text{ kg/m}^3$*



# SMART ROCK DESIGN AND PROTOTYPING

- **Final Design of Smart Rocks (Cont.)**
  - ***Internal Configuration (APSS)***
    - ✓ *Monitored along the river bank*
    - ✓ *Measurement station in South or North pole of the magnet*
    - ✓ *Rapid convergence and high accuracy of APSS location*
    - ✓ *However, easy affected by ferromagnetic substance*



**APSS Model of Smart Rocks**

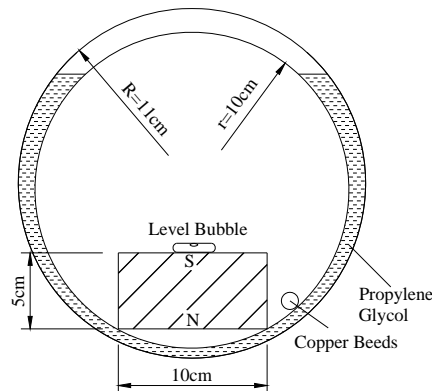


# SMART ROCK DESIGN AND PROTOTYPING

- **Final Design of Smart Rocks (Cont.)**

- **Internal Configuration (APUS)**

- ✓ **Automatically Pointing Upward System (APUS)**
    - ✓ **Magnetometer set on the bridge deck, measurement station in south pole of the magnet**
    - ✓ **Two poles of magnet aligned with vertical sensor of the magnetometer**
    - ✓ **Gravity-orientated direction, reduces the degree of freedom, less effect by ferromagnetic substance**



(a) Schematic View (b) Prototype Smart Rock (c) Balanced Magnet  
APUS Model of Smart Rocks



# SMART ROCK DESIGN AND PROTOTYPING

- **Final Design of Smart Rocks (Cont.)**

- ***Design Details***

A cylindrical magnet placed inside an organic glass ball (inside ball), an outside organic glass ball, liquid filled in between two balls, and a concrete shell encasement. *Inside ball floating inside the outside ball.*

- ✓ ***Diameter Selection***

- *Magnet: 10 cm in diameter and 5cm in height*
      - *Inside ball: 20 cm based on availability of casting molds, smart rock size and floating requirement*
      - *Outside ball: 21 cm based on sufficient spacing for lubrication*

- ✓ ***Liquid Selection***

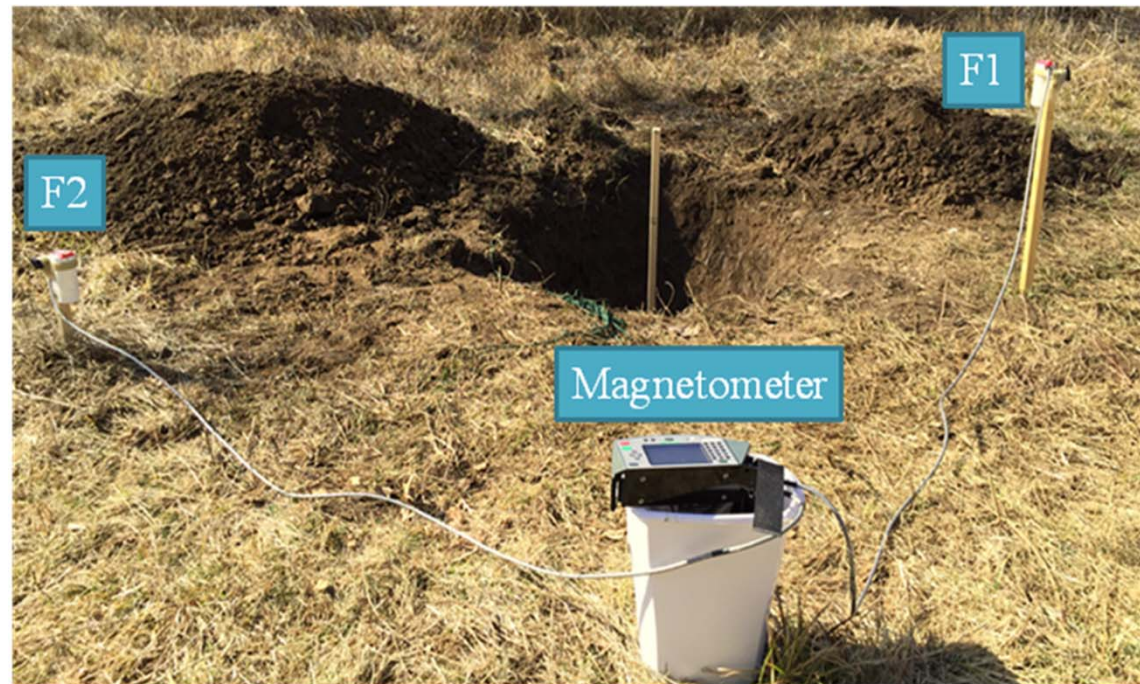
- *No friction force on the inside ball*
      - *Nontoxicity requirement*
      - *Density greater than 850 kg/m<sup>3</sup>*
      - *Therefore, propylene glycol with 1040kg/m<sup>3</sup>*





# SMART ROCK DESIGN AND PROTOTYPING

- **Final Design of Smart Rocks (Cont.)**
  - *Effect of Deposit Resetting*



**Overall Arrangement of Resetting Tests**



# SMART ROCK DESIGN AND PROTOTYPING

- **Final Design of Smart Rocks (Cont.)**
  - *Effect of Deposit Resetting (Cont.)*

(a) 0.0 m



(b) 0.5 m



(c) 1.0 m



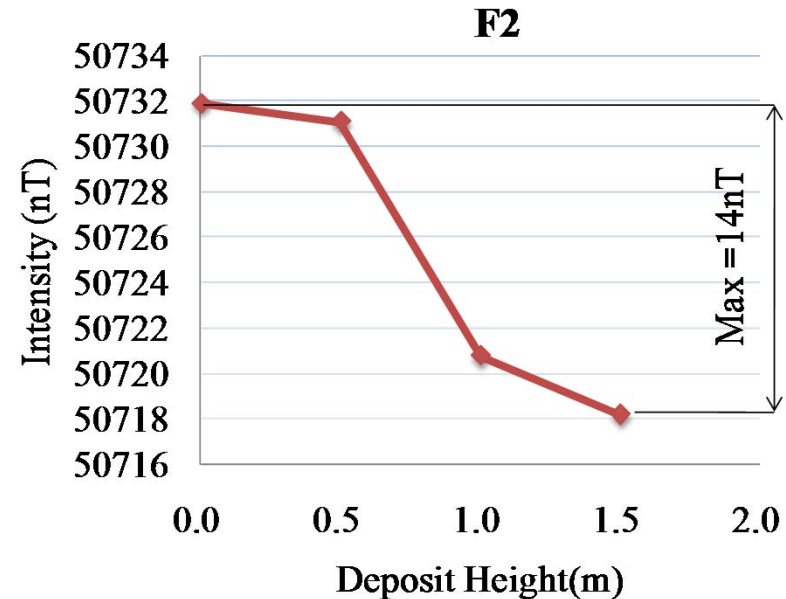
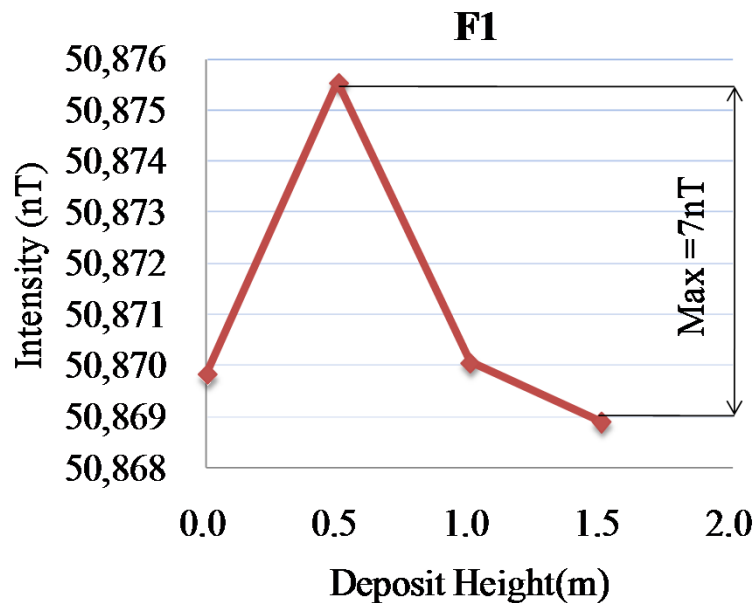
(d) 1.5 m





# SMART ROCK DESIGN AND PROTOTYPING

- Final Design of Smart Rocks (Cont.)
  - *Effect of Deposit Resetting (Cont.)*



The intensity variations at different heights for measurement F1 and F2



# SMART ROCK DESIGN AND PROTOTYPING

- **Final Design of Smart Rocks**
  - ***Effect of Steel Reinforcement***
    - ✓ *Bubble in the center 10m away from the bridge pier*
    - ✓ *Bubble slightly deviated, indicating an inclination angle of less than  $0.5^\circ$*



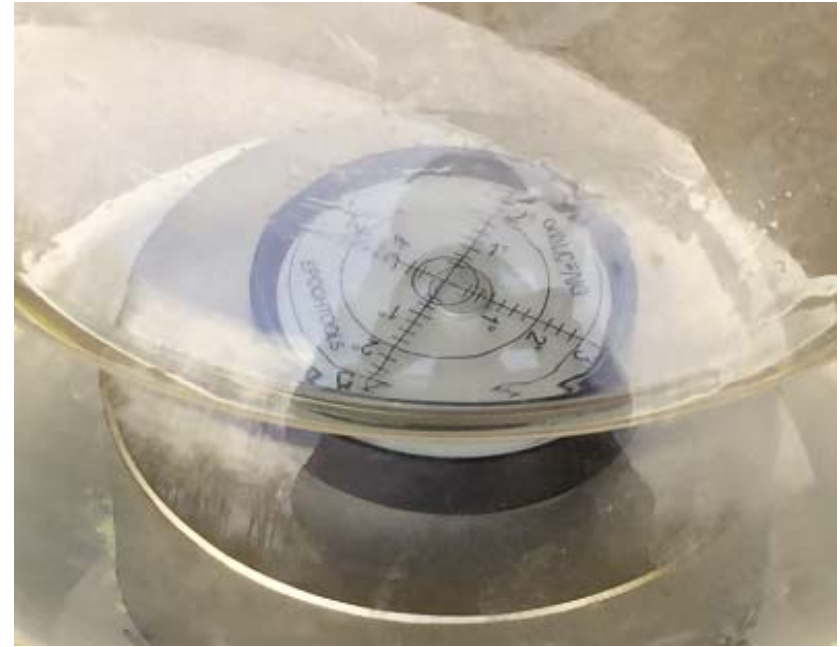
**The Prototype APUS Placed next to a Bridge Pier**





# SMART ROCK DESIGN AND PROTOTYPING

- **Final Design of Smart Rocks**
  - *Effect of Steel Reinforcement (Cont.)*
    - ✓ *Little effect on the localization of the APUS*



**The Prototype APUS Placed on a Bridge Foundation**





# SMART ROCK DESIGN AND PROTOTYPING

- **Prototyping with Concrete Encasement**
  - *Spherical concrete encasement*
  - *25-cm-diameter mold*
  - *Close to the target value of 1530 kg/m<sup>3</sup>*
  - *Total density is 1520 kg/m<sup>3</sup>, appropriated for Highway 1 Waddell Creek Bridge, Highway 9 Kings Creek Bridge, US-63 Gasconade River Bridge and I-44 Roubidoux Creek Bridge*



**A Prototype Smart Rock**

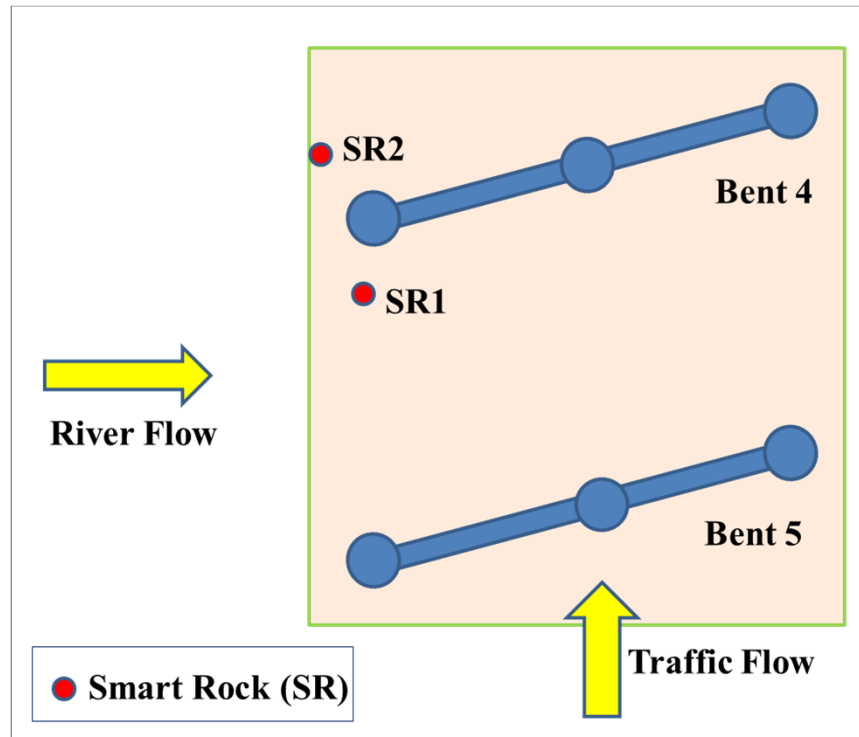


# FUTURE TASKS

- **Deployment of Smart Rocks**

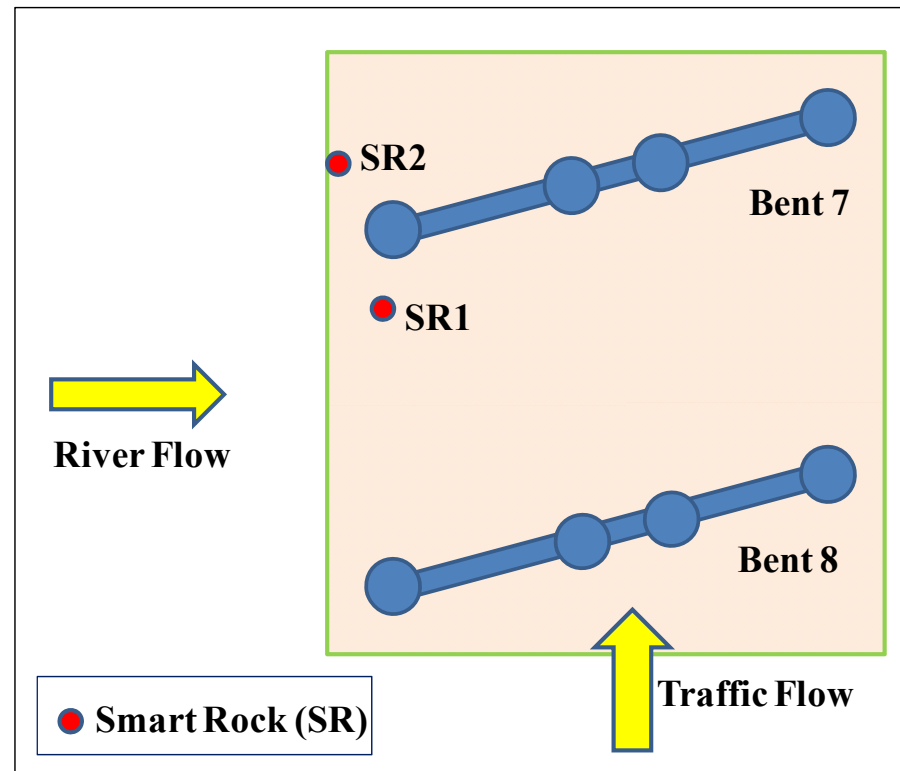
- **US 63 Gasconade River Bridge**

- ✓ *The exact location of smart rocks (SR1, SR2) around Pier 4 will be determined when deployed.*



# FUTURE TASKS

- **Deployment of Smart Rocks (Cont.)**
  - **I-44 Roubidoux Creek Bridge**
    - ✓ *The exact location of smart rocks (SR1, SR2) around Pier 7 will be determined when deployed.*

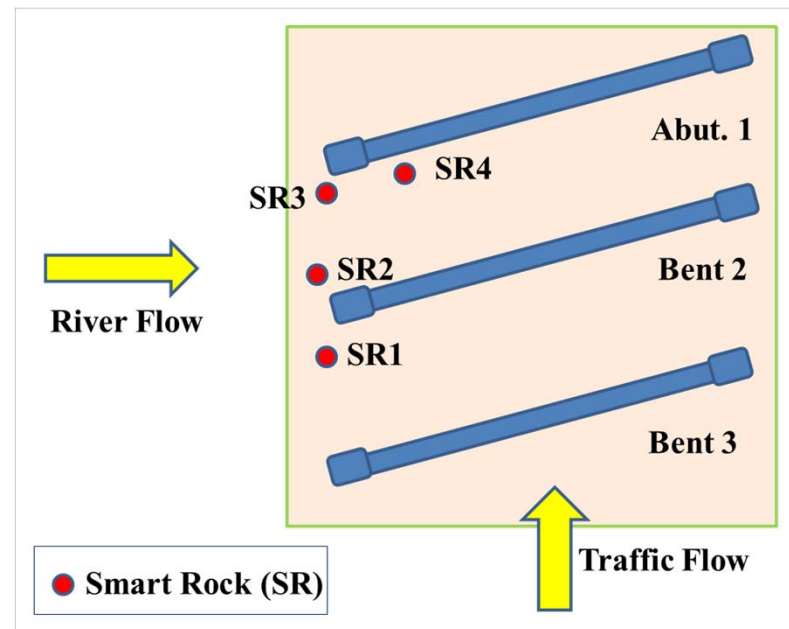


# FUTURE TASKS

- **Deployment of Smart Rocks (Cont.)**

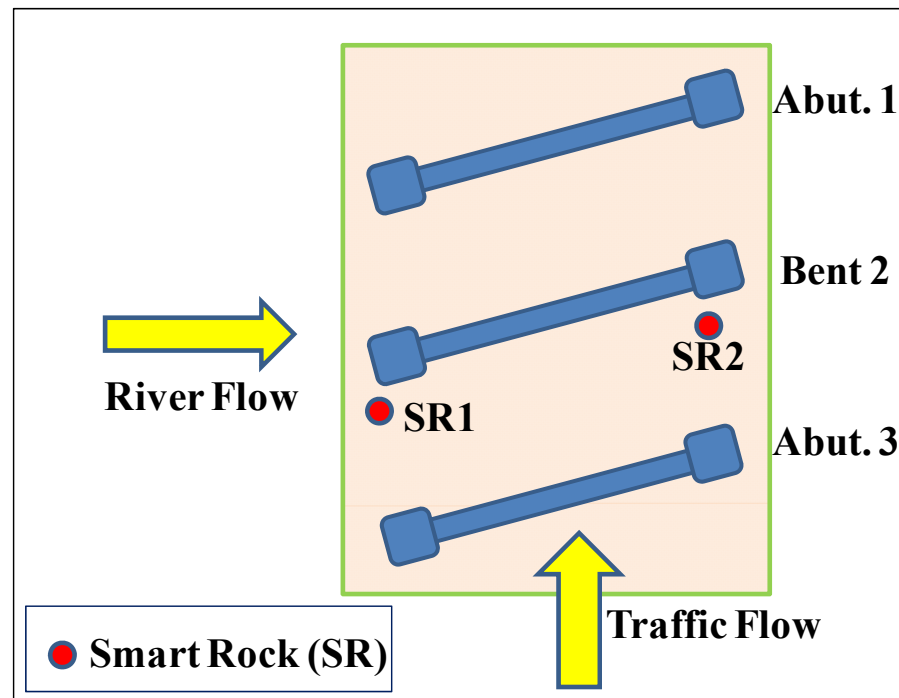
- **Highway 1 Waddell Creek Bridge**

- ✓ *The exact location of smart rocks (SR1, SR2) around Pier 2 will be determined when deployed.*
- ✓ *SR3 and SR4 around abutment 1 are deployed to monitor the effectiveness of the riprap measure.*



# FUTURE TASKS

- **Deployment of Smart Rocks (Cont.)**
  - **Highway 9 Kings Creek Bridge**
    - ✓ *Main flow goes through between Bent 2 and Abut.3.*
    - ✓ *SR1 and SR2 are in the upstream and downstream sides of the pier at Bent 2.*





# FUTURE TASKS

- **Measurement Plan**

- **Concept and Practice on Bridge Deck**

- ✓ *Wood Frame with sensor*
    - ✓ *X- Longitudinal direction of the bridge*
    - ✓ *Y- Transverse direction of the bridge*
    - ✓ *Z- Upward*
    - ✓ *Movement in X-, Y-, and Z- directions*
    - ✓ *Measurement points distribute above the smart rocks*
    - ✓ *However, wood frame swung under wind load makes the difficulty to get the correct measurements.*

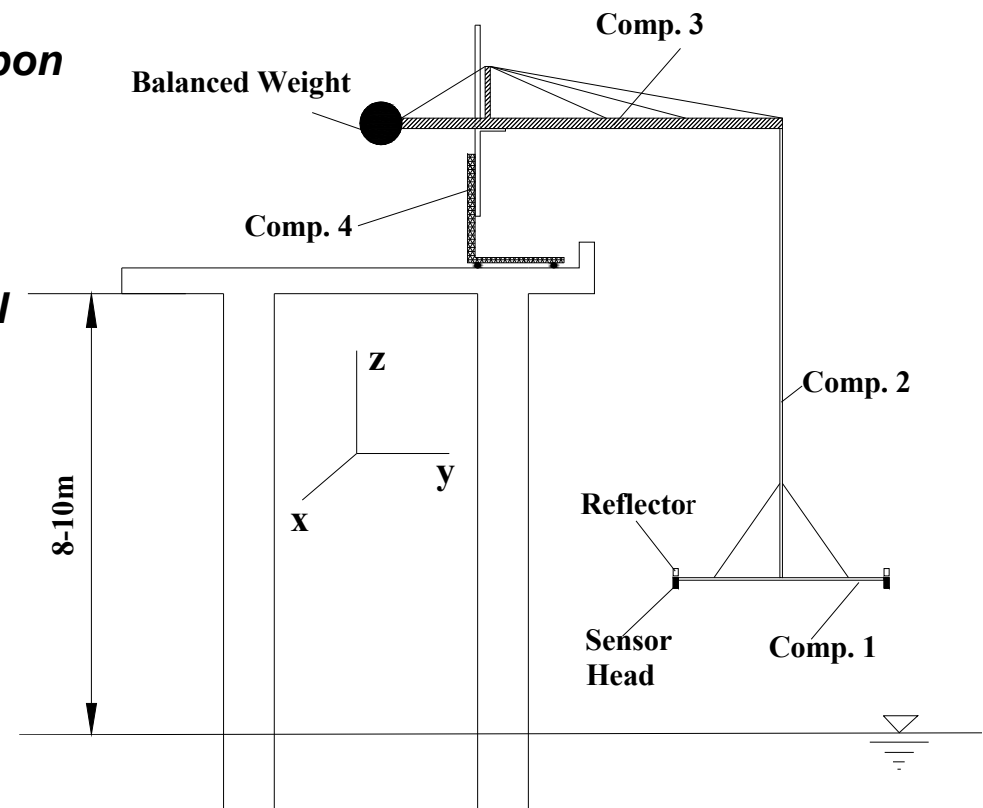


# FUTURE TASKS

- **Measurement Plan (Cont.)**

- **Prototype Light Frame for Rapid Assembling on Site**

- ✓ *Comp. 1 – Lower horizontal beam for fixing sensor (carbon fiber)*
    - ✓ *Comp. 2 – Vertical beam (carbon fiber)*
    - ✓ *Comp. 3 – Higher Horizontal beam (Aluminum alloy)*
    - ✓ *Comp. 4 – Manual forklift*
    - ✓ *X-, Y-, and Z- direction movement by forklift*

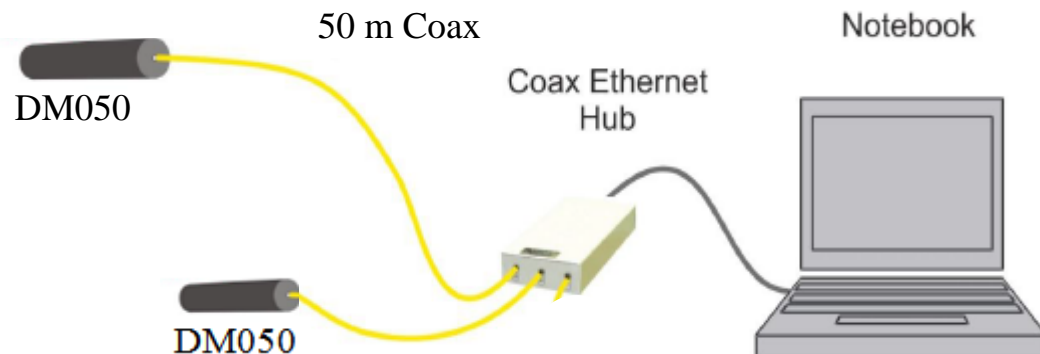


# FUTURE TASKS

- **Measurement Plan (Cont.)**

- **A 3 Axis Magnetometer**

- ✓ *STL Digital Magnetometer (Type DM050) – Measure X-, Y- and Z-component of any magnetic field*
    - ✓ *50 meter Coax cable for power and data transmission*
    - ✓ *Interface – Coax Ethernet Hub for connection of up to 3 magnetometers*
    - ✓ *STL GradMag software installed in a Notebook for full controlling of measurement, data acquisition and viewer*





# ANY COMMENTS?

